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The incremental harmonic balance method for nonlinear vibration of axially moving beams

K.Y. Sze^{a,*}, S.H. Chen^b, J.L. Huang^b

^aDepartment of Mechanical Engineering, The University of Hong Kong, Pokfulam Road, Hong Kong SAR, PR China ^bDepartment of Applied Mechanics and Engineering, Zhongshan University Guangzhou, 510275, PR China

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Abstract

In this paper, the incremental harmonic balance (IHB) method is formulated for the nonlinear vibration analysis of axially moving beams. The Galerkin method is used to discretize the governing equations. A high-dimensional model that can take nonlinear model coupling into account is derived. The forced response of an axially moving strip with internal resonance between the first two transverse modes is studied. Particular attention is paid to the fundamental, superharmonic and subharmonic resonance as the excitation frequency is close to the first, second or one-third of the first natural frequency of the system. Numerical results reveal the rich and interesting nonlinear phenomena that have not been presented in the existent literature on the nonlinear vibration of axially moving media. © 2004 Elsevier Ltd. All rights reserved.

1. Introduction

Axially moving systems can be found in a wide range of engineering problems which arise in industrial, civil, mechanical, electronic and automotive applications. Magnetic tapes, power transmission belts and band saw blades are examples where an axial transport of mass is associated with a transverse vibration.

Analytical models for axially moving systems have been extensively studied in the last few decades. The vast literature on axially moving material vibration has been reviewed by Wickert

^{*}Corresponding author. Tel.: +852-2859-2637; fax: +852-2858-5415.

E-mail address: kysze@hku.hk (K.Y. Sze).

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and Mote [1] up to 1988. More recently, the problem of axially moving media has been tackled in the analysis of particular aspects such as different solution techniques, discretization approaches, modeling aspects and nonlinear phenomena, see the review in Ref. [2]. Most of these studies addressed the problem of constant axial transport velocity and constant axial tension. Wickert and Mote [3] studied the transverse vibration of axially moving strings and beams using an eigenfunction method. They also used the Green function method to study the dynamic response of an axially moving string loaded with a traveling periodic suspended mass [4]. Wickert [5] presented a complete study of the nonlinear vibrations and bifurcations of moving beams using the Krylov–Bogoliubov–Mitropolsky asymptotic method. Chakraborty et al. [6,7] investigated both free and forced responses of a traveling beam using nonlinear complex normal modes. Pellicano and Vestroni [2] studied the dynamic behavior of an axially moving beam using a highdimensional discrete model obtained by the Galerkin procedure. Al-Jawi et al. [8–10] investigated the effects of tension disorder, inter-span coupling and translational speed on the confinement of the natural modes of free vibration through the exact, the perturbation and the Galerkin approaches. By analytical and numerical means, Chen [11] studied the natural frequencies and stability of an axially traveling string in contact with a stationary load system which contains parameters such as dry friction, inertia, damping and stiffness. Riedel and Tan [12] studied the coupled and forced responses of an axially moving strip with internal resonance. The method of multiple scales is used to perform the perturbation analysis and to determine the frequency response numerically for both low and high speeds.

There are papers devoted to the analysis of the dynamic behavior of traveling systems with time-dependent axial velocity or with time-dependent axial tension force. Pakdemirli et al. [13] conducted a stability analysis using Floquet theory for sinusoidal transporting velocity function. They also investigated the principal parametric resonance and the combination of resonances for an axially accelerating string using the method of multiple scales [14]. Mockensturm et al. [15] applied the Galerkin procedure and the perturbation method of Krylov-Bogoliubov-Mitropolsky to examine the stability and limit cycles of parametrically excited and axially moving strings in the presence of tension fluctuations. Zhang and Zu [16,17] employed the method of multiple scales to study the nonlinear vibration of parametrically excited moving belts. Suweken and Van Horssen [18] used a two time-scales perturbation method to approximate the solutions of a conveyor belt with a low and time-varying velocity. Öz and Pakdemirli [19,20] also applied the method of multiple scales to study the vibration of an axially moving beam with time-dependent velocity. Fung and Chang [21] employed the finite difference method with variable grid for numerical calculation of string/slider nonlinear coupling system with time-dependent boundary condition. Ravindra and Zhu [22] studied the low-dimensional chaotic response of axially accelerating continuum in the supercritical regime. Moon and Wickert [23] performed an analytical and experimental study on the response of a belt excited by pulley eccentricities. Pellicano et al. [24] studied the primary and parametric nonlinear resonance of a power transmission belt by experimental and theoretical analysis.

In this paper, the incremental harmonic balance (IHB) method is applied to analyze the nonlinear vibration of axially moving systems. The IHB method was originally presented by Lau and Cheung [25], Cheung and Lau [26] and Lau et al. [27]. It has been developed and successfully applied to the analysis of periodic nonlinear structural vibrations and the related problems. However, none of these applications is related to axially moving systems. This paper starts with an

introduction on the essence of the IHB method. Using the method, some particular cases of the axially moving beam problem are effectively treated. As a matter of fact, the generalization of the IHB method to other moving media and other nonlinear vibration problems is simple and straightforward.

2. Equations of motion

The governing equations of two dimensional, planar motion of an axially moving beam can be derived using Hamilton's Principle. Following a similar derivation to that of Wickert [5], one obtains the following coupled nonlinear dimensionless equations of motion:

$$(u_{,tt} + 2vu_{,xt} + v^2u_{,xx}) - v_1^2(u_{,x} + \frac{1}{2}w_{,x}^2)_{,x} = 0,$$
(1)

$$(w_{,tt} + 2vw_{,xt} + v^2w_{,xx}) - \{[1 + v_1^2(u_{,x} + \frac{1}{2}w_{,x}^2)]w_{,x}\}_{,x} + v_f^2w_{,xxxx} = 0,$$
(2)

where

$$u = U/L, \quad w = W/L, \quad x = X/L, \quad t = T\sqrt{P/\rho A L^2},$$
 (3)

$$v = V/\sqrt{P/\rho A}, \quad v_1 = \sqrt{EA/P}, \quad v_f = \sqrt{EI/PL^2}.$$
 (4)

Here, X and Z are respectively the longitudinal and transverse coordinates of the beam, U and W are respectively the longitudinal and transverse displacements, V is the axial speed, T denotes time and P is the axial tension, see Fig. 1. Properties of the beam include the beam length L, the cross-section area A, the second moment of area I, the mass density ρ and the elastic modulus E. Lastly, $(u_{,tt}, w_{,tt})$, $(2vu_{,xt}, 2vw_{,xt})$ and $(v^2u_{,xx}, v^2w_{,xx})$ are respectively the local, Coriolis and centripetal acceleration vectors. In particular, V, P, L, A, I, ρ and I are constants whereas P must be nonzero. The boundary conditions for a hinged-hinged beam are

$$u(0,t) = u(1,t) = 0,$$
(5)

$$w(0,t) = w(1,t) = 0, \quad w_{,xx}(0,t) = w_{,xx}(1,t) = 0.$$
 (6)



Fig. 1. Schematic diagram for an axially moving beam and its coordinate system, where F denotes the force acting per unit length of the beam.

3. Separation of variables

In the nonlinear vibration analysis of continuous systems, the variables u(x, t) and w(x, t) in the partial differential equations (1) and (2) with boundary conditions (5) and (6) are usually separated. The use of eigenfunctions as a complete basis is often the choice because of their promising accuracy and convergence. Unfortunately, the eigenfunctions of traveling beams are complex and dependent on the speed. In the present study, we assume the following separable solutions in terms of admissible functions:

$$u(x,t) = \sum_{j=1}^{N} q_{j}^{u}(t) \sin j\pi x,$$
(7)

$$w(x,t) = \sum_{j=1}^{N} q_{j}^{w}(t) \sin j\pi x.$$
(8)

After substituting Eqs. (7) and (8) into Eqs. (1) and (2), the Galerkin procedure leads to the following set of N + M second-order ordinary differential equations

$$\sum_{j=1}^{N} M_{ij}^{u} \ddot{q}_{j}^{u} + \sum_{j=1}^{N} C_{ij}^{u} \dot{q}_{j}^{u} + \sum_{j=1}^{N} K_{ij}^{u} q_{j}^{u} + \sum_{j=1}^{N} \sum_{k=1}^{M} K_{ijk}^{w} q_{k}^{w} q_{j}^{w} = 0 \quad \text{for } i = 1, 2, \dots, N,$$
(9)

$$\sum_{j=1}^{M} M_{ij}^{w} \ddot{q}_{j}^{w} + \sum_{j=1}^{M} C_{ij}^{w} \dot{q}_{j}^{w} + \sum_{j=1}^{M} K_{ij}^{w} q_{j}^{w} + \sum_{j=1}^{M} \sum_{k=1}^{N} K_{ijk}^{u} q_{k}^{u} q_{j}^{w} + \sum_{j=1}^{M} \sum_{k=1}^{M} \sum_{l=1}^{M} K_{ijkl}^{w} q_{l}^{w} q_{k}^{w} q_{j}^{w} = 0$$

for $i = 1, 2, ..., M$, (10)

where the dot above a variable denotes its derivative with respect to the nondimensional time t,

$$\begin{split} M_{ij}^{u} &= M_{ij}^{w} = \int_{0}^{1} \sin i\pi x \sin j\pi x \, \mathrm{d}x = \frac{1}{2} \delta_{ij}, \\ K_{ij}^{u} &= -(v^{2} - v_{1}^{2})j^{2}\pi^{2} \int_{0}^{1} \sin i\pi x \sin j\pi x \, \mathrm{d}x = -\frac{1}{2}(v^{2} - v_{1}^{2})j^{2}\pi^{2} \delta_{ij}, \\ C_{ij}^{u} &= C_{ij}^{w} = 2vj\pi \int_{0}^{1} \sin i\pi x \cos j\pi x \, \mathrm{d}x = \begin{cases} 4ijv/(i^{2} - j^{2}), & i \neq j \text{ and } (i+j) \text{ is even,} \\ 0, & \text{otherwise,} \end{cases} \\ K_{ijk}^{w} &= v_{1}^{2}jk^{2}\pi^{3} \int_{0}^{1} \sin i\pi x \cos j\pi x \sin k\pi x \, \mathrm{d}x = v_{1}^{2}jk^{2}\pi^{3}I_{scs}(i,j,k), \\ K_{ij}^{w} &= [v_{fj}^{2}j^{4}\pi^{4} - (v^{2} - 1)j^{2}\pi^{2}] \int_{0}^{1} \sin i\pi x \sin j\pi x \, \mathrm{d}x = \frac{1}{2}[v_{fj}^{2}j^{4}\pi^{4} - (v^{2} - 1)j^{2}\pi^{2}]\delta_{ij}, \end{split}$$

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$$\begin{split} K_{ijk}^{u} &= v_{1}^{2} j k^{2} \pi^{3} \int_{0}^{1} \sin i\pi x \cos j\pi x \sin k\pi x \, dx + v_{1}^{2} k j^{2} \pi^{3} \int_{0}^{1} \sin i\pi x \cos k\pi x \sin j\pi x \, dx, \\ &= v_{1}^{2} j k^{2} \pi^{3} I_{scs}(i,j,k) + v_{1}^{2} k j^{2} \pi^{3} I_{scs}(i,k,j), \\ K_{ijkl}^{w} &= \frac{3}{2} v_{1}^{2} j^{2} k l \pi^{4} \int_{0}^{1} \sin i\pi x \sin j\pi x \cos k\pi x \cos l\pi x \, dx = \frac{3}{2} v_{1}^{2} j^{2} k l \pi^{4} I_{sscc}(i,j,k,l), \\ I_{scs}(i,j,k) &= \frac{1}{2} [I_{cc}(i-k,j) - I_{cc}(I+k,j)], \end{split}$$

 $I_{sscc}(i,j,k,l) = \frac{1}{4}[I_{cc}(i-j, k+l) + I_{cc}(i-j, k-l) - I_{cc}(i+j, k+l) - I_{cc}(i+j, k-l)],$

$$I_{cc}(i,j) = \int_{0}^{1} \cos i\pi x \cos j\pi x \, \mathrm{d}x = \begin{cases} 0, & i \neq j, \\ 1/2, & i = j \neq 0, \\ 1, & i = j = 0. \end{cases}$$

Eqs. (9) and (10) can be written in matrix–vector form as

$$\mathbf{M}^{u}\ddot{\mathbf{q}}^{u} + \mathbf{C}^{u}\dot{\mathbf{q}}^{u} + \mathbf{K}^{u}\mathbf{q}^{u} + \mathbf{K}^{w}_{2}(\mathbf{q}^{w})\mathbf{q}^{w} = \mathbf{0},$$
(11)

$$\mathbf{M}^{w}\ddot{\mathbf{q}}^{w} + \mathbf{C}^{w}\dot{\mathbf{q}}^{w} + \mathbf{K}^{w}\mathbf{q}^{w} + \mathbf{K}_{2}^{u}(\mathbf{q}^{u})\mathbf{q}^{w} + \mathbf{K}_{3}^{w}(\mathbf{q}^{w})\mathbf{q}^{w} = \mathbf{0},$$
(12)

where $\mathbf{q}^{u} = [\mathbf{q}_{1}^{u}, \mathbf{q}_{2}^{u}, \dots, \mathbf{q}_{N}^{u}]^{\mathrm{T}}$ and $\mathbf{q}^{w} = [\mathbf{q}_{1}^{w}, \mathbf{q}_{2}^{w}, \dots, \mathbf{q}_{M}^{w}]^{\mathrm{T}}$. The entries of matrices $\mathbf{M}^{u}, \mathbf{M}^{w}, \mathbf{C}^{u}, \mathbf{C}^{w}, \mathbf{K}^{u}$ and \mathbf{K}^{w} are respectively $M_{ij}^{u}, M_{ij}^{w}, C_{ij}^{u}, C_{ij}^{w}, K_{ij}^{u}$ and K_{ij}^{w} . Furthermore, the entries of matrices $\mathbf{K}_{2}^{w}(\mathbf{q}^{w}), \mathbf{K}_{2}^{u}(\mathbf{q}^{u})$ and $\mathbf{K}_{3}^{w}(\mathbf{q}^{w})$ are respectively

$$K_{2_{ij}}^{w} = \sum_{k=1}^{M} k_{ijk}^{w} q_{k}^{w}, \quad K_{2_{ij}}^{u} = \sum_{k=1}^{N} K_{ijk}^{u} q_{k}^{u} \quad \text{and} \quad K_{3_{ij}}^{w} = \sum_{k=1}^{M} \sum_{l=1}^{M} K_{ijkl}^{w} q_{k}^{w} q_{l}^{v}.$$

Eqs. (11) and (12) can be grouped as

$$\mathbf{M}\ddot{\mathbf{q}} + \mathbf{C}\dot{\mathbf{q}} + \mathbf{K}\mathbf{q} + \mathbf{K}_2(\mathbf{q})\mathbf{q} + \mathbf{K}_3(\mathbf{q})\mathbf{q} = \mathbf{0},$$
(13)

where

$$\mathbf{q} = [\mathbf{q}^{u}, \mathbf{q}^{w}]^{\mathrm{T}}, \ \mathbf{M} = \begin{bmatrix} \mathbf{M}^{u} & \mathbf{0} \\ \mathbf{0} & \mathbf{M}^{w} \end{bmatrix}, \ \mathbf{C} = \begin{bmatrix} \mathbf{C}^{u} & \mathbf{0} \\ \mathbf{0} & \mathbf{C}^{w} \end{bmatrix},$$
$$\mathbf{K} = \begin{bmatrix} \mathbf{K}^{u} & \mathbf{0} \\ \mathbf{0} & \mathbf{K}^{w} \end{bmatrix}, \ \mathbf{K}_{2} = \begin{bmatrix} \mathbf{0} & \mathbf{K}_{2}^{w} \\ \mathbf{0} & \mathbf{K}_{2}^{u} \end{bmatrix}, \ \mathbf{K}_{3} = \begin{bmatrix} \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{K}_{3}^{w} \end{bmatrix}.$$

For the forced response of the system, an excitation term can be added to the right-hand side of Eq. (13), i.e.

$$\mathbf{M}\ddot{\mathbf{q}} + \mathbf{C}\dot{\mathbf{q}} + \mathbf{K}\mathbf{q} + \mathbf{K}_2(\mathbf{q})\mathbf{q} + \mathbf{K}_3(\mathbf{q})\mathbf{q} = \mathbf{F} \cos n\omega t$$
(14)

in which ω is the nondimensional excitation frequency whose physical counterpart is $\omega \sqrt{P/\rho AL^2}$.

4. IHB formulation

In this section, the IHB method is formulated to solve Eq. (14). With the new dimensionless time variable τ defined as

$$\tau = \omega t, \tag{15}$$

Eq. (14) becomes

$$\omega^2 \mathbf{M} \mathbf{q}'' + \omega \mathbf{C} \mathbf{q}' + [\mathbf{K} + \mathbf{K}_2(\mathbf{q}) + \mathbf{K}_3(\mathbf{q})] \mathbf{q} = \mathbf{F} \cos n\tau$$
(16)

in which prime denotes differentiation with respect to τ .

The first step of the IHB method is the incremental procedure. Let q_{j0} and ω_0 denote a state of vibration; the neighboring state can be expressed by adding the corresponding increments as

$$\omega = \omega_0 + \Delta \omega, \quad q_j = q_{j0} + \Delta q_j, \tag{17}$$

where j = 1, 2, ..., m and m = N + M.

Substituting Eqs. (17) into Eq. (16) and neglecting the higher-order incremental terms, one obtains the following linearized incremental equation in matrix–vector form:

$$\omega_0^2 \mathbf{M} \Delta \mathbf{q}'' + \omega_0 \, \mathbf{C} \Delta \mathbf{q}' + (\mathbf{K} + \mathbf{K}_2^* + 3\mathbf{K}_3) \Delta \mathbf{q} = \mathbf{R} - (2\omega_0 \mathbf{M} \mathbf{q}_0'' + \mathbf{C} \mathbf{q}_0') \Delta \omega, \tag{18}$$

$$\mathbf{R} = \mathbf{F} \cos n\tau - \{\omega_0^2 \mathbf{M} \mathbf{q}_0'' + \omega_0 \mathbf{C} \mathbf{q}_0' + [\mathbf{K} + \mathbf{K}_2(\mathbf{q}_0) + \mathbf{K}_3(\mathbf{q}_0)] \mathbf{q}_0\}$$
(19)

in which

$$\mathbf{q}_0 = [q_{10}, \ q_{20}, \dots, q_{m0}]^{\mathrm{T}}, \ \Delta \mathbf{q} = [\Delta q_1, \ \Delta q_2, \dots, \Delta q_m]^{\mathrm{T}},$$
$$\mathbf{K}_2^* = \begin{bmatrix} 0 & 2\mathbf{K}_2^w \\ \mathbf{K}_2^+ & \mathbf{K}_2^u \end{bmatrix}, \ K_{2ij}^+ = \sum_{k=1}^M K_{ijk}^u q_k^w$$

and ${\bf R}$ is a residual/corrective vector that goes to zero when the (numerical) solution is exact.

The second step of the IHB method is the harmonic balance procedure. Let

$$q_{j0} = \sum_{k=1}^{n_c} a_{jk} \cos(k-1)\tau + \sum_{k=1}^{n_s} b_{jk} \sin k\tau = \mathbf{C}_s \mathbf{A}_j,$$
(20)

$$\Delta q_j = \sum_{k=1}^{n_c} \Delta a_{jk} \cos(k-1)\tau + \sum_{k=1}^{n_s} \Delta b_{jk} \sin k\tau = \mathbf{C}_s \Delta \mathbf{A}_j$$
(21)

where

$$\mathbf{C}_{s} = [1, \cos \tau, \dots, \cos (n_{c} - 1)\tau, \sin \tau, \dots, \sin n_{s} \tau],$$
$$\mathbf{A}_{j} = [a_{j1}, a_{j2}, \dots, a_{jn_{c}}, b_{j1}, b_{j2}, \dots, b_{jn_{s}}]^{\mathrm{T}},$$
$$\Delta \mathbf{A}_{j} = [\Delta a_{j1}, \Delta a_{j2}, \dots, \Delta a_{jn_{c}}, \Delta b_{j1}, \Delta b_{j2}, \dots, \Delta b_{jn_{s}}]^{\mathrm{T}}.$$

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Then, \mathbf{q}_0 and $\Delta \mathbf{q}$ can be expressed in terms of the Fourier coefficient vector $\mathbf{A} = [\mathbf{A}_1, \mathbf{A}_2, \dots, \mathbf{A}_m]^T$ and its increment $\Delta \mathbf{A} = [\Delta \mathbf{A}_1, \Delta \mathbf{A}_2, \dots, \Delta \mathbf{A}_m]^T$ as

$$\mathbf{q}_0 = \mathbf{S}\mathbf{A}, \quad \Delta \mathbf{q} = \mathbf{S}\Delta \mathbf{A} \tag{22}$$

in which $S = \text{diag.} [C_s, C_s, \dots, C_s]$. Substituting Eqs. (22) into Eq. (18) and applying the Galerkin procedure for one cycle, one obtains the following set of linear equations in terms of ΔA and $\Delta \omega$:

$$\bar{\mathbf{K}}_{mc}\Delta \mathbf{A} = \bar{\mathbf{R}} - \bar{\mathbf{R}}_{mc}\Delta\omega,\tag{23}$$

where

$$\overline{\mathbf{K}}_{mc} = \int_{0}^{2\pi} \mathbf{S}^{\mathrm{T}} [\omega_{0}^{2} \mathbf{M} \mathbf{S}'' + \omega_{0} \mathbf{C} \mathbf{S}' + (\mathbf{K} + \mathbf{K}_{2}^{*} + 3\mathbf{K}_{3}) \mathbf{S}] \, \mathrm{d}\tau,$$
$$\overline{\mathbf{R}} = \int_{0}^{2\pi} \mathbf{S}^{\mathrm{T}} \{ \mathbf{F} \cos n\tau - [\omega_{0}^{2} \mathbf{M} \mathbf{S}'' + \omega_{0} \mathbf{C} \mathbf{S}' + (\mathbf{K} + \mathbf{K}_{2} + \mathbf{K}_{3}) \mathbf{S}] \} \, \mathrm{d}\tau \, \mathbf{A},$$
$$\overline{\mathbf{R}}_{mc} = \int_{0}^{2\pi} \mathbf{S}^{\mathrm{T}} (2\omega_{0} \mathbf{M} \mathbf{S}'' + \mathbf{C} \mathbf{S}') \, \mathrm{d}\tau \, \mathbf{A}.$$

The solution process begins with a guessed solution. The nonlinear frequency–amplitude response curve is then solved point-by-point by incrementing the frequency ω or incrementing a component of the coefficient vector **A**. The Newton–Raphson iterative method can be employed.

5. Numerical calculations

To illustrate the power of the IHB method, some numerical examples are presented in this section. If we take N=M=1 in Eqs. (7) and (8), i.e. only one longitudinal mode and one transverse mode are taken, then Eqs. (9) and (10) become

$$\ddot{q}_1^u - (v^2 - v_1^2)\pi^2 q_1^u = 0, (24)$$

$$\ddot{q}_1^w + (v_f^2 \pi^2 - v^2 + 1)\pi^2 q_1^w + \frac{3}{8} v_1^2 \pi^4 (q_1^w)^3 = 0.$$
⁽²⁵⁾

Obviously, the longitudinal vibration is linear and the transverse vibration is nonlinear. Moreover, they are not coupled. Eq. (25) is the famous Duffing equation and its nonlinear dynamic characteristics have been thoroughly investigated by many researchers. Letting N = M = 2, i.e. two longitudinal and two transverse modes are considered, Eqs. (9) and (10) become

$$\ddot{q}_1^u + \mu_1 \dot{q}_2^w - (v^2 - v_1^2)\pi^2 q_1^u + v_1^2 \pi^3 q_1^w q_2^w = 0,$$
(26)

$$\ddot{q}_{2}^{u} + \mu_{2}\dot{q}_{1}^{u} - 4(v^{2} - v_{1}^{2})\pi^{2}q_{2}^{u} + \frac{1}{2}v_{1}^{2}\pi^{3}(q_{1}^{w})^{2} = 0,$$
(27)

$$\ddot{q}_{1}^{w} + \mu_{1} \dot{q}_{2}^{w} + k_{11} q_{1}^{w} + k_{12} q_{1}^{w} (q_{2}^{w})^{2} + k_{13} (q_{1}^{w})^{3} + v_{1}^{2} \pi^{3} (q_{1}^{u} q_{2}^{w} + q_{2}^{u} q_{1}^{w}) = 0,$$
(28)

$$\ddot{q}_{2}^{w} + \mu_{2} \dot{q}_{1}^{w} + k_{21} q_{2}^{w} + k_{22} q_{2}^{w} (q_{1}^{w})^{2} + k_{23} (q_{2}^{w})^{3} + v_{1}^{2} \pi^{3} q_{1}^{u} q_{1}^{w} = 0,$$
⁽²⁹⁾

where

$$\mu_1 = -16v/3, \quad k_{11} = (v_f^2 \pi^2 - v^2 + 1)\pi^2, \quad k_{12} = 3v_1^2 \pi^4, \quad k_{13} = k_{12}/8,$$
$$\mu_2 = -\mu_1, \quad k_{21} = 4(4v_f^2 \pi^2 - v^2 + 1)\pi^2, \quad k_{22} = k_{12}, \quad k_{23} = 2k_{12}.$$

Riedel and Tan [12] investigated the forced transverse response of an axially moving strip using the method of multiple scales. With reference to the typical parameters of a belt drive system given in Ref. [12], we shall assume

$$v_1^2 = 1124$$
, $v_f^2 = 0.0015$ and $v = 0.6$

throughout this section. The natural frequencies can be approximated with the linear undamped natural frequencies by dropping the nonlinear and damping terms in Eqs. (26)–(29). In this light, the linear natural frequencies are estimated to be 2.54 and 5.25 for the transverse vibration, and 105.31 and 210.62 for the longitudinal vibration. The natural frequencies of the transverse vibration are far away from that of the longitudinal vibration so that the coupled effect between them should be weak. Hence, we will focus on the forced transverse response of the moving beam by neglecting the effect of the longitudinal vibration in the following study. By setting q_i^u s to zero and by incorporating the force terms, Eqs. (28) and (29) become

$$\ddot{q}_1^w + \mu_1 \dot{q}_2^w + k_{11} q_1^w + k_{12} q_1^w (q_2^w)^2 + k_{13} (q_1^w)^3 = F_1 \cos \Omega t,$$
(30)

$$\ddot{q}_{2}^{w} + \mu_{2} \dot{q}_{1}^{w} + k_{21} q_{2}^{w} + k_{22} q_{2}^{w} (q_{1}^{w})^{2} + k_{23} (q_{2}^{w})^{3} = F_{2} \cos \Omega t,$$
(31)

where Ω is the forcing frequency. By dropping the nonlinear terms, the linear natural frequencies can be solved from the following equation:

$$\omega^4 - (k_{11} + k_{21} - \mu_1 \mu_2)\omega^2 + k_{11}k_{21} = 0.$$
(32)

For the chosen v_1 and v_f , Fig. 2 plots the ratio of the second natural frequency ω_{20} to the first natural frequency ω_{10} versus the axial speed v. For a nonlinear system with cubic nonlinearity, the internal resonance usually occurs when the natural frequency $\omega_{20} \approx 3\omega_{10}$. It can be found from Fig. 2 that $\omega_{20}/\omega_{10} \approx 3$ when $v_1^2 = 1124$, $v_f^2 = 0.0015$ and v = 0.6. With this value of v, $\omega_{10} = 2.11641$ and $\omega_{20} = 6.3109$.

5.1. Fundamental resonance at Ω near ω_{10}

In order to obtain the fundamental resonance when the forcing frequency Ω is near the first natural frequency ω_{10} , one should take $F_2 = 0$ in Eq. (31). In the following calculation, we take $F_1 = 0.03$ and $n_c = n_s = 4$. As Eqs. (30) and (31) do not contain quadratic nonlinear terms, q_1^w and q_2^w should not contain even harmonic terms and, thus, can be expressed as

$$q_1^w = A_{11} \cos(\tau + \phi_{11}) + A_{13} \cos 3(\tau + \phi_{13}) + \dots,$$
(33)

$$q_2^w = A_{21} \cos\left(\tau + \phi_{21}\right) + A_{23} \cos 3\left(\tau + \phi_{23}\right) + \dots$$
(34)

where $\tau = \Omega t$ and ϕ denote phase difference. The fundamental response is expressed mainly by the $\Omega - A_{11}, \Omega - A_{13}, \Omega - A_{21}$ and $\Omega - A_{23}$ curves which are shown in Figs. 3(a–d), respectively. A_{11} and A_{13} , defined in Eq. (33), are respectively the amplitudes of the first and the third harmonic

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Fig. 2. Ratio of transverse natural frequencies versus speed for $v_1^2 = 1124$ and $v_f^2 = 0.0015$.

terms in the first variable q_1^w . Meanwhile, A_{21} and A_{23} , defined in Eq. (34), are respectively the amplitudes of the first and the third cosine harmonic terms of the second variable q_2^w . To facilitate the understanding of the relation between forced and free vibration, the free vibration backbone curves for A_{11} , A_{13} , A_{21} and A_{23} are also plotted. It can be seen that $|A_{11}| \ge |A_{13}|$ and $|A_{23}| \ge |A_{21}|$. Therefore, the first and third harmonic terms are the major modes of q_1^w and q_2^w , respectively.

One can note the internal resonance on the amplitudes of the responding modes in Figs. 3a and d. Both A_{11} and A_{23} possess three solutions. Their first solutions $A_{11}^{(1)}$ and $A_{23}^{(1)}$ are of different phases, and represent the in-phase and out-of-phase responses, respectively. Starting from P₁'s in the respective figures, both $|A_{11}^{(1)}|$ and $|A_{23}^{(1)}|$ increase with Ω to the turning points P₂'s. Before P₂'s, $|A_{11}^{(1)}| > |A_{23}^{(1)}|$ and thus the major mode of q_1^w is the responding mode. At P₂'s, $|A_{11}^{(1)}|$ jumps down and $|A_{23}^{(1)}|$ jumps up. Afterwards, $|A_{11}^{(1)}|$ drops and $|A_{23}^{(1)}|$ rises slowly. The responding modes switch from the major mode of q_2^w . Though the frequency pertinent to the $A_{23}^{(1)}$ is $3\Omega t$, $A_{23}^{(1)}$ here is not triggered by $F_2\cos(3\Omega t)$ which acts on q_2^w . Rather, $A_{23}^{(1)}$ is induced by the nonlinear effect intrinsic to the dynamic system. This phenomenon is typically known as internal resonance. Noticeably, the frequencies at P₂'s are close to the first natural frequency ω_{10} or the exchange of the responding mode occurs at the vicinity of ω_{10} .

The responding mode occurs at the vicinity of ω_{10} . The second solutions of A_{11} and A_{23} are respectively $A_{11}^{(2)}$ and $A_{23}^{(2)}$ which are of the same phase. $A_{11}^{(2)}$ and $A_{23}^{(2)}$ start from P₄'s and terminate at P₆'s via turning points P₅'s. In the course, $|A_{11}^{(2)}|$ increases gradually but $|A_{23}^{(2)}|$ decreases gradually. In other words, the responding mode transits gradually from the major mode of q_2^w back to that of q_1^w . The third solutions of A_{11} and A_{23} are respectively $A_{11}^{(3)}$ and $A_{23}^{(3)}$ which are again of the same phase. $A_{11}^{(3)}$ and $A_{23}^{(3)}$ start from P₇'s and terminate at P₉'s via turning points P₈'s. In the course, both $|A_{11}^{(3)}|$ and $|A_{23}^{(2)}|$ drop gradually. There is no exchange of responding mode as in the first solutions ($A_{11}^{(1)}$ and $A_{23}^{(1)}$) and second solutions ($A_{11}^{(2)}$ and $A_{23}^{(2)}$).

The internal resonance pattern for A_{13} and A_{21} revealed in Figs. 3b and c are similar to that of A_{11} and A_{23} . However, A_{13} and A_{21} are only the minor modes of respectively q_1^w and q_2^w as $|A_{13}| \approx |A_{21}| \ll |A_{11}| \approx |A_{23}|$.

To the best knowledge of the authors, the internal resonance phenomenon portrayed in Figs. 3a and d bodies have not been reported in the literature of axially moving bodies and nonlinear



Fig. 3. Fundamental resonance as $\Omega \approx \omega_{10}$: —, denotes forced response curve and - -, denotes backbone curve.

vibration. Nevertheless, the phenomenon is very much similar to the internal resonance previously reported for nonlinear vibration of clamped-hinged beams [28] and thin plates with one edge clamped and the opposite edge hinged [29]. Though they arises from different sets of governing equations and are solved by different methods, they share the common cubic nonlinearity feature and $\omega_{20} \approx 3\omega_{10}$.

Fig. 4 shows the forced response curves of the Duffing equation as obtained by adding the excitation term $F_1 \cos \Omega t$ to the right-hand side of Eq. (25). Comparing Fig. 4 with Fig. 3a, one can discover the important difference between the characteristics of the one- and the two-degree-of-freedom systems. In the single-degree-of-freedom system, $A_{11}^{(1)}$ and $A_{11}^{(2)}$ in Fig. 3a becomes $A_1^{(1)}$ in Fig. 4. The turning point P₂ does not exist and there is no internal resonance for the in-phase response curve. On the other hand, the out-of-phase responses $A_{11}^{(3)}$ and $A_{1}^{(3)}$ in the two figures exhibit the same characteristic.

5.2. Superharmonic resonance at Ω near $\omega_{10}/3$

In order to obtain the superharmonic resonance as Ω near $\omega_{10}/3$, one should again take a nonzero F_1 and a zero F_2 in Eqs. (30) and (31). Furthermore, we set $F_1 = 0.03$ and $n_c = n_s = 4$. The solution of q_1^w and q_2^w are again taken in the form of Eqs. (33) and (34). Results show $|A_{13}| \gg |A_{11}|$ and $|A_{23}| \gg |A_{21}|$. In other words, the response is dominated by the third harmonic terms whose frequency is three times of the excitation frequency. Consequently, these resonance



Fig. 4. Forced response curve of Duffing equation as $\Omega \approx \omega_{10}$: —, denotes forced response curve and - -, denotes backbone curve.

are called superharmonic resonance which are expressed by the $\Omega - A_{13}$ and $\Omega - A_{23}$ curves in Figs. 5a and b, respectively. The superharmonic responses of $\Omega - A_{13}$ and $\Omega - A_{23}$ show the same characteristics which are similar to that of the fundamental resonance of the single-degree-of-freedom system in Fig. 4. The sharp turns of the frequency-amplitude response curves are responsible for the jump phenomenon.

5.3. Fundamental resonance at Ω near ω_{20}

In order to obtain the fundamental resonance as Ω nears the second natural frequency ω_{20} , one should take a zero F_1 and a nonzero F_2 in Eqs. (30) and (31). Furthermore, we take $F_2 = 0.03$ and $n_c = n_s = 4$. The solutions of q_1^w and q_2^w are also taking the form of Eqs. (33) and (34). Numerical results show that $|A_{11}| \gg |A_{13}|$ and $|A_{21}| \gg |A_{23}|$. In other words, the response is dominated by the first harmonic terms whose frequency is the same as the excitation frequency. Therefore, these responses are termed as fundamental resonances. They are expressed by the curves $\Omega - A_{21}$ and $\Omega - A_{11}$ in Figs. 6a and b, respectively. The characteristic of A_{11} and A_{21} is similar to that of the single-degree-of-freedom system shown in Fig. 4.

5.4. Subharmonic resonance at Ω near ω_{20}

Besides the fundamental resonance, there is another kind of resonance named subharmonic resonance that occurs when $\Omega \approx \omega_{20}$. To obtain the subharmonic resonance, one should take a zero F_1 in Eq. (30) and a nonzero F_2 on the third harmonic term in Eq. (31). Let $\tau = \Omega$ $t = 3\tau_1$, the solution of q_1^w and q_2^w in the present case are taken to be

$$q_j^w = \sum_{k=1}^{n_c} a_{jk} \cos((2k-1)\tau_1) + \sum_{k=1}^{n_s} b_{jk} \sin((2k-1)\tau_1) \quad \text{for } j = 1, 2.$$
(35)



Fig. 5. Superharmonic resonance as $\Omega \approx \omega_{10}/3$: —, denotes forced response curve and - -, denotes backbone curve.

Again, we take $f_2 = 0.03$ and $n_c = n_s = 4$. Numerical results show that the coefficients a_{jk} and b_{jk} (j = 1, 2, k = 3, 4) are near to zero. Hence, the assumed solutions are simplified to be

$$q_1^{W} = A_{11} \cos \frac{1}{3} (\tau + \phi_{11}) + A_{13} \cos (\tau + \phi_{13}) + \cdots,$$
(36)

$$q_2^{w} = A_{21} \cos \frac{1}{3} (\tau + \phi_{21}) + A_{23} \cos (\tau + \phi_{23}) + \cdots$$
(37)

Results show that $|A_{11}| \gg |A_{13}|$ and $|A_{21}| \gg |A_{23}|$. In other words, the response is dominated by the first harmonic terms whose frequency is one-third of the excitation frequency. Hence, these resonances are called subharmonic resonances which are expressed by the $\Omega - A_{11}$ and $\Omega - A_{21}$ curves in Figs. 7a and b, respectively. In other words, the excitation at a high frequency may produce significant responses in the low-frequency modes and, particularly, the fundamental



Fig. 6. Fundamental resonance as $\Omega \approx \omega_{20}$: —, denotes forced response curve and - -, denotes backbone curve.

mode. Figs. 7a–d show how complicated the solution can be when $\Omega \approx \omega_{20}$. Some interesting phenomena are noted as follows:

- 1. There are two solutions in the subharmonic resonance as $\Omega \approx \omega_{20}$.
- 2. $|A_{11}|$ in Fig. 7a can be as large as $6 |A_{21}|$ in Fig. 7b, $10 |A_{23}|$ in Fig. 7d and even $20 |A_{13}|$ in Fig. 7c. Clearly, the subharmonic response is dominated by A_{11} or the subharmonic term of the first variable.
- Double roots are encountered for A₁₃ and A₂₃ as shown in Figs. 7c and d. In other words, A⁽¹⁾₁₃ = A⁽²⁾₁₃, A⁽¹⁾₂₃ = A⁽²⁾₂₃.
 There is no jump phenomenon in all subharmonic resonance.

Lastly, it is worth pointing out that one can calculate another set of solutions when $A_{11} =$ $A_{21} = 0$. As $\Omega \approx \omega_{20}$, the pertinent response curves $\Omega \approx A_{13}$ and $\Omega \approx A_{23}$ express the fundamental resonance and are similar to the $\Omega - A_{11}$ and $\Omega - A_{21}$ curves in Figs. 6a and b.



Fig. 7. Subharmonic resonance as $\Omega \approx \omega_{20}$: —, denotes forced response curve and - -, denotes backbone curve.

6. Concluding remarks

- 1. The Incremental Harmonic Balance (IHB) method has been shown to be a straightforward, efficient and reliable method for treating the nonlinear vibration of axially moving systems.
- 2. The forced response of an axially moving beam with internal resonance between the first two transverse modes is studied. In the presence of internal resonance due to the coupling of the two modes, numerical results reveal that excitation at a frequency close to the fundamental frequency can produce a significant response at a higher harmonic frequency. Conversely, excitation at a frequency close to a higher harmonic frequency may produce a significant response at a lower harmonic frequency and, in particular, at the fundamental frequency. The observed internal resonances are rich and complicated. To the best knowledge of the authors, the observations have not been reported in the literature on nonlinear vibration analysis of axially moving media.
- 3. Stability analysis of the periodic response has not been studied here. It will be discussed in a separate paper.

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